

Random Eigenvalue Problems in Structural Dynamics: An Experimental Investigation

S. Adhikari, A. Srikantha Phani and D. A. Pape

School of Engineering, Swansea University, Swansea, UK Email: S.Adhikari@swansea.ac.uk URL: http://engweb.swan.ac.uk/~adhikaris



Palm Springs, CA, 6th May 2009



- A Brief Overview of Random Eigenvlaue Problems
- Random Eigenvalues of a Fixed-Fixed Beam
- Random Eigenvalues of a cantilever plate
- System Model and Experimental Setup
- Experimental methodology
- Eigenvalue Statistics
 - Experimental results
 - Monte Carlo simulation
- Conclusions & future directions





Many structural dynamic systems are manufactured in a production line (nominally identical sys-









Complex aerospace system can have millions of degrees of freedom and significant 'errors' and/or 'lack of knowledge' in its numerical (Finite Element) model

Palm Springs, CA, 6th May 2009

Random Eigenvalue Problems in Structural Dynamics – p.4/30



(a) parametric uncertainty - e.g., uncertainty in geometric parameters, friction coefficient, strength of the materials involved;
(b) model inadequacy - arising from the lack of scientific knowledge about the model which is a-priori unknown;
(c) experimental error - uncertain and unknown error percolate into the model when they are calibrated against experimental results;

(d) computational uncertainty - e.g, machine precession, error tolerance and the so called 'h' and 'p' refinements in finite element analysis, and

(e) model uncertainty - genuine randomness in the model such as uncertainty in the position and velocity in quantum mechanics, deterministic chaos.



Overview of Random Eigenvalue Problems

EVP of Undamped or proportionally damped systems:

$$\mathbf{K}\boldsymbol{\phi}_j = \lambda_j \mathbf{M}\boldsymbol{\phi}_j \tag{1}$$

 λ_j : Eigenvalue (natural frequency squared) ϕ_j : Eigenvector (modeshape) M &K are symmetric and P.D random matrices $\Rightarrow \lambda_j$ real and positive.

$$\mathbf{M} = \overline{\mathbf{M}} + \delta \mathbf{M} \quad \text{and} \quad \mathbf{K} = \overline{\mathbf{K}} + \delta \mathbf{K}. \tag{2}$$

(•): Nominal (deterministic) of of (•)
$$\delta(\bullet)$$
: Random parts of (•).



$\mathbf{M} = \overline{\mathbf{M}} + \delta \mathbf{M}$ and $\mathbf{K} = \overline{\mathbf{K}} + \delta \mathbf{K}$.

- $\delta \mathbf{M}$ and $\delta \mathbf{K}$ are zero-mean random matrices.
- Small randomness assumption that preserve symmetry and P.D of M and M.
- No assumptions on the type of randomness: need not be Gaussian, for example
- Fixed-Fixed beam with random placement of equal masses gives $\delta M \neq 0 \ \delta K = 0$
- Cantilever plate with random placement of random oscillators gives $\delta M \neq 0 \ \delta K \neq 0$







The test rig for the fixed-fixed beam Actuator: Shaker, Sensors: Accelerometers



Palm Springs, CA, 6th May 2009

Random Eigenvalue Problems in Structural Dynamics – p.8/30





Attached masses (magnets) at random locations. 12 masses, each weighting 2g, are used.

Palm Springs, CA, 6th May 2009

Fixed-Fixed Beam: Properties

Beam Properties	Numerical values	
Length (L)	1200 mm	
Width (b)	40.06 mm	
Thickness (t_h)	2.05 mm	
Mass density (ρ)	7800 Kg/m 3	
Young's modulus (E)	$2.0 \times 10^5 \text{ MPa}$	
Total weight	0.7687 Kg	
Material and geometric properties of the beam		

pulse rate: 20s & pulse width: 0.01s. Eliminate input uncertainties.

brass plate (2g) takes impact.

Palm Springs, CA, 6th May 2009

Random Eigenvalue Problems in Structural Dynamics -p.11/30

Experiments: Protocol

- Arrange the masses along the beam at random locations (computer generated)
- Measure impulse response at: 23 cm (Point1) 50 cm (Point2, also the actuation point) and 102 cm (Point3) from the left end of the beam in a 32 channel LMSTM system
- Transform to frequency domain to estimate frequency response function (FRF).
- Curvefit the FRF to estimate the natural frequencies ω_n and damping factors Q_n
 - Rational Fraction Polynomial (RFP) method
 - Nonlinear Leastsquares method

Calculate the statistics of natural frequencies

Experiments: FRF at point 2 (the driving point FRF, 50 c²Ec cm from the left end)

Ensemble Mean

Left: RFP; Right: Nonlinear Leastsquares

C²EC

Palm Springs, CA, 6th May 2009

Random Eigenvalue Problems in Structural Dynamics -p.16/30

Standard Deviation

Left: RFP; Right: Nonlinear Least-squares

C²EC

Palm Springs, CA, 6th May 2009

Left: RFP; Right: Nonlinear Least-squares

The test rig for the cantilever plate: Front View

Palm Springs, CA, 6th May 2009

Random Eigenvalue Problems in Structural Dynamics - p.19/30

Palm Springs, CA, 6th May 2009

Random Eigenvalue Problems in Structural Dynamics - p.20/30

Plate Properties	Numerical values
Length (L)	998 mm
Width (b)	530 mm
Thickness (t_h)	3.0 mm
Mass density (ρ)	7800 Kg/m 3
Young's modulus (E)	$2.0 imes 10^5 \text{ MPa}$
Total weight	12.38 Kg

Table 1: Material and geometric properties of the cantilever plate considered for the experiment

C²FC

Attached oscillators at random locations. The spring stiffness varies

Palm Springs, CA, 6th May 2009

Properties of Attached Oscillators

Oscillator Number	Spring stiffness ($ imes 10^4$ N/m)	Natural Frequency (Hz)
1	1.6800	59.2060
2	0.9100	43.5744
3	1.7030	59.6099
4	2.4000	70.7647
5	1.5670	57.1801
6	2.2880	69.0938
7	1.7030	59.6099
8	2.2880	69.0938
9	2.1360	66.7592
10	1.9800	64.2752

Table 2: Stiffness of the springs and natural frequency of the oscillators used to simulate unmodelled dynamics (the mass of the each oscillator is 121.4g).

Swansea University Prifysgol Abertawe

Ø

C²EC

- Attach the oscillators at random locations (computer generated)
- Measure impulse response at: Point 1: (4,6), Point 2: (6,11), Point 3: (11,3), Point 4: (14,14), Point 5: (18,2), Point 6: (21,10)
- Transform to frequency domain to estimate frequency response function (FRF).
- Curvefit the FRF to estimate the natural frequencies ω_n and damping factors Q_n
 - Rational Fraction Polynomial (RFP) method
 - Nonlinear Leastsquares method

Calculate the statistics of natural frequencies

Left: RFP; Right: Nonlinear Leastsquares

C²EC

Palm Springs, CA, 6th May 2009

Random Eigenvalue Problems in Structural Dynamics – p.27/30

Left: RFP; Right: Nonlinear Least-squares

Palm Springs, CA, 6th May 2009

Left: RFP; Right: Nonlinear Least-squares

Conclusions

- The ensemble statistics such as mean and standard deviation for natural frequencies vary with the spatial location of the measured FRFs and the type of the system identification technique chosen to estimate the natural frequencies.
- Whilst a reasonable predictions for the mean and the standard deviations may be obtained using the Monte Carlo Simulation, higher moments, and hence the pdfs can be significantly different.
- In some cases, the differences in pdfs arising from different points and different identification methods can be more than those obtained from the Monte Carlo Simulation.

